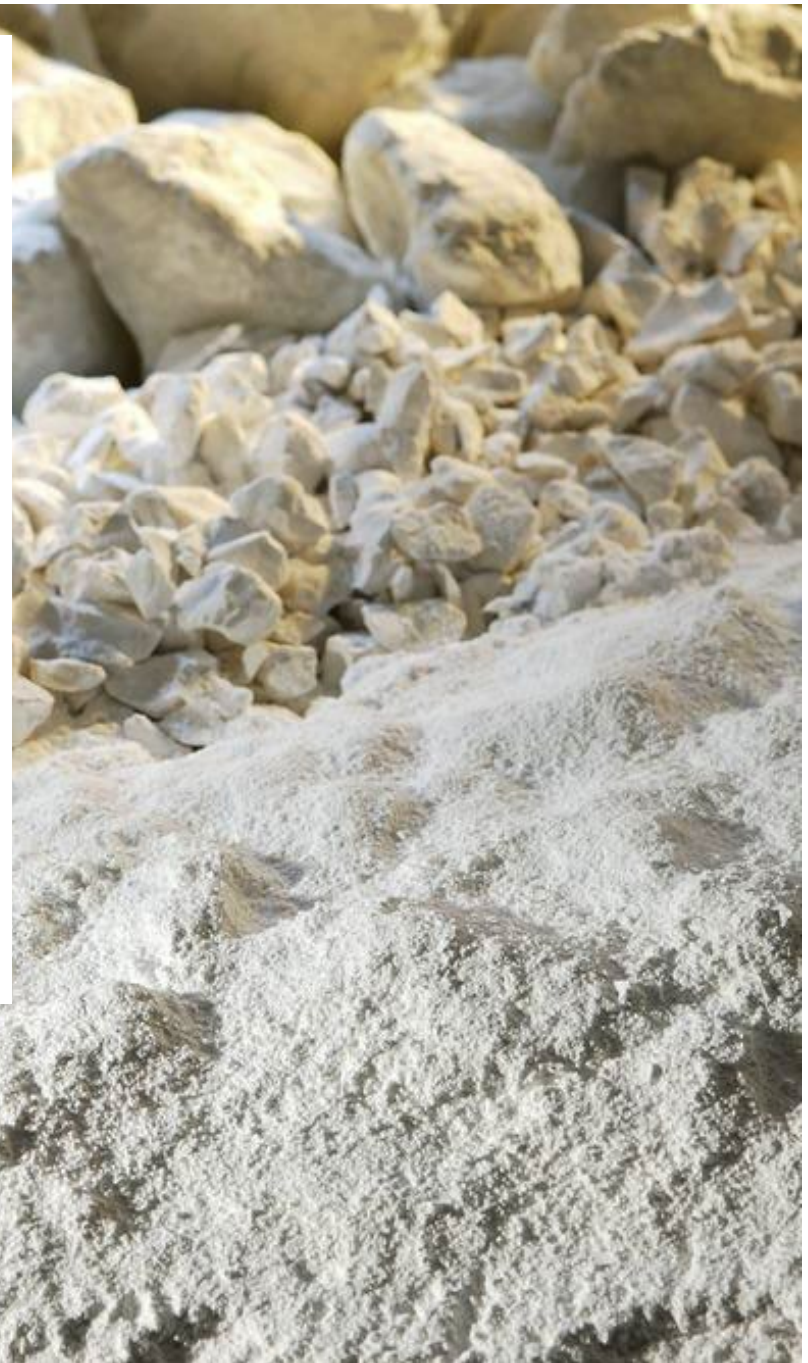




# THE IMPORTANCE OF A GOOD FOUNDATION FOR OPTIMUM PAVEMENT PERFORMANCE

TxAPA MAPS Conference 2023  
Waco, Texas  
May 16, 2023

Larry Peirce



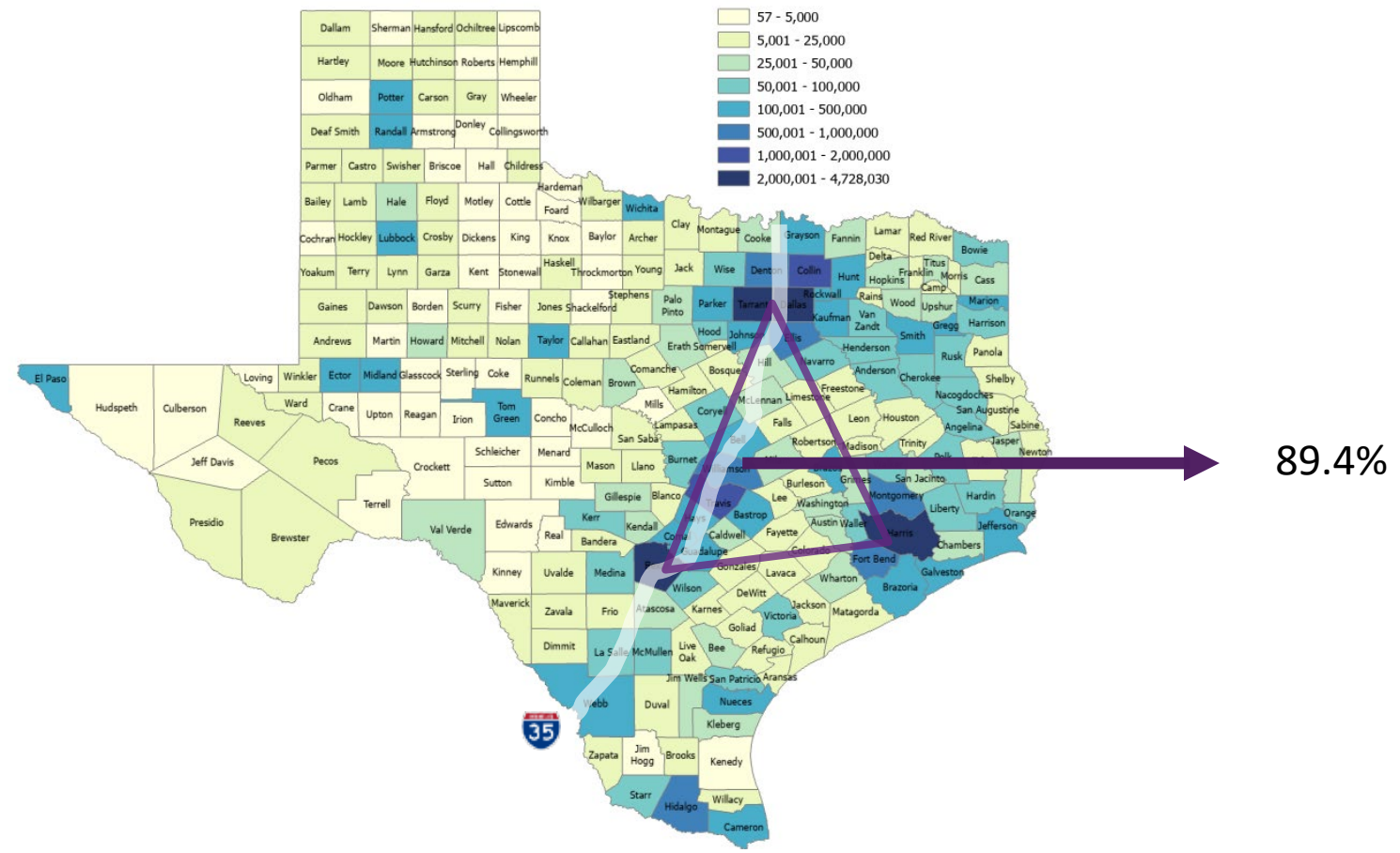
# OPTIMUM PAVEMENT PERFORMANCE

## Depends On A Combination Of Factors

- Understanding of Your Objectives
- Choosing the Right Construction Materials
- Properly Designing Those Materials
- Ensuring That You Have Created an Adequate Foundation
- Treatment (Stabilization) of The Base and Subgrade Soils



# 2021 Estimated Population, Texas Counties

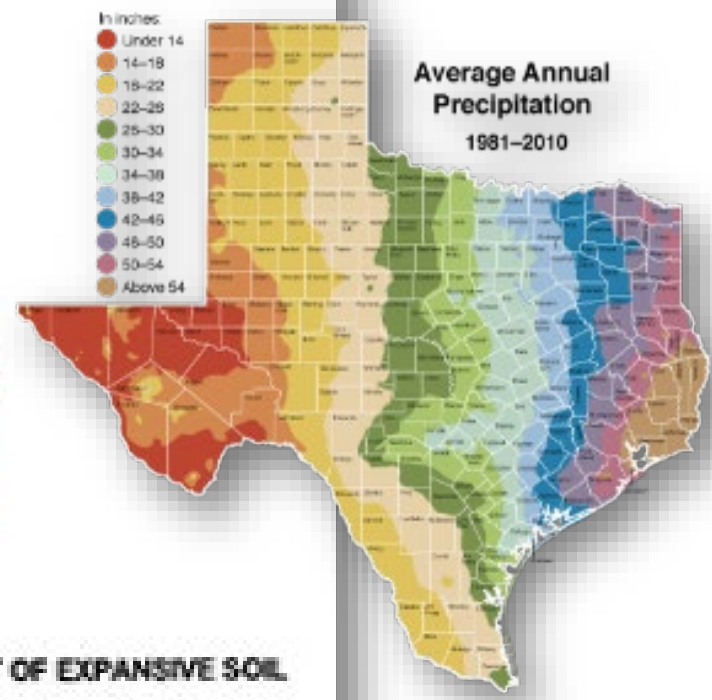
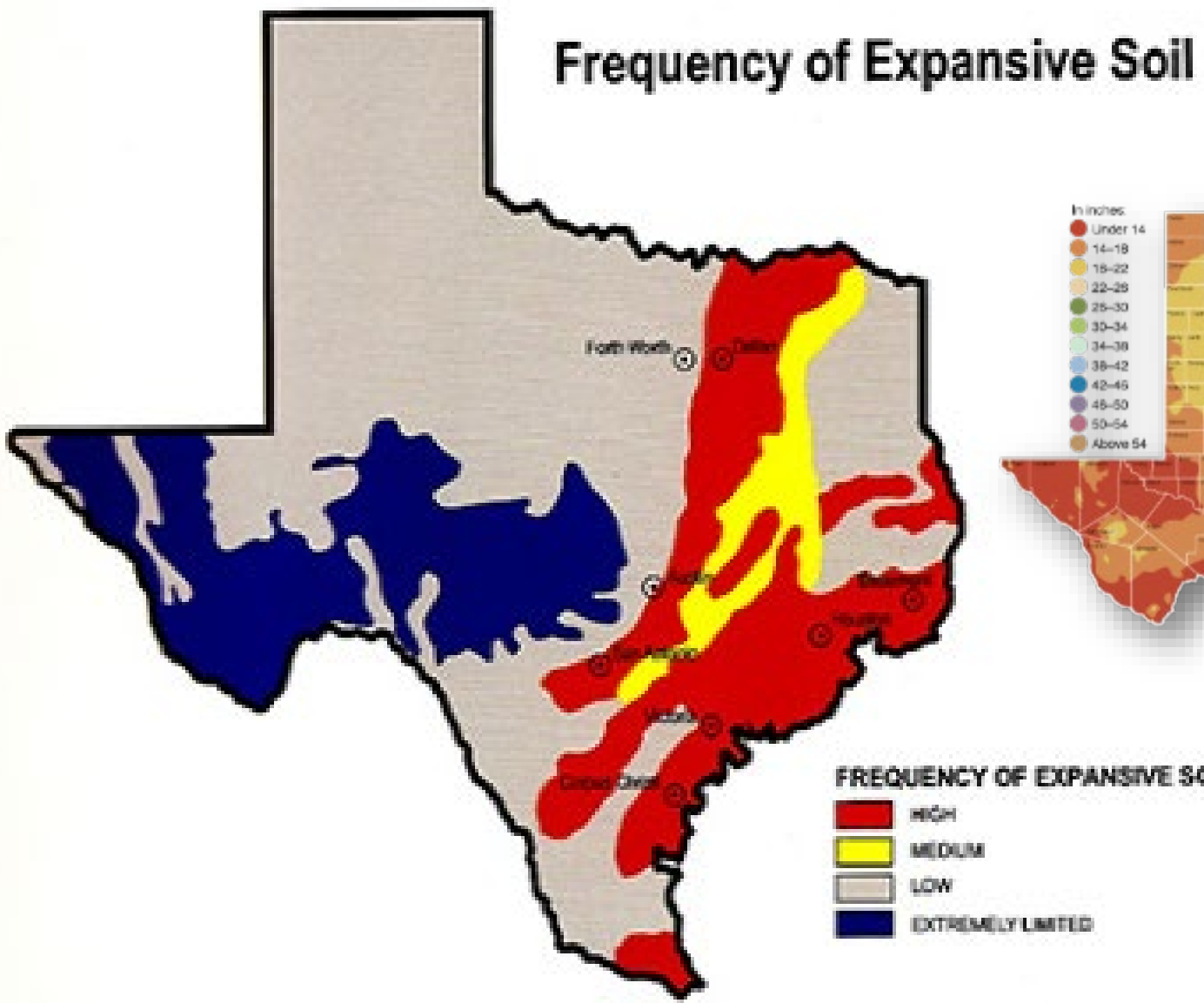


Source: US Census Bureau, 2021 Population Estimates

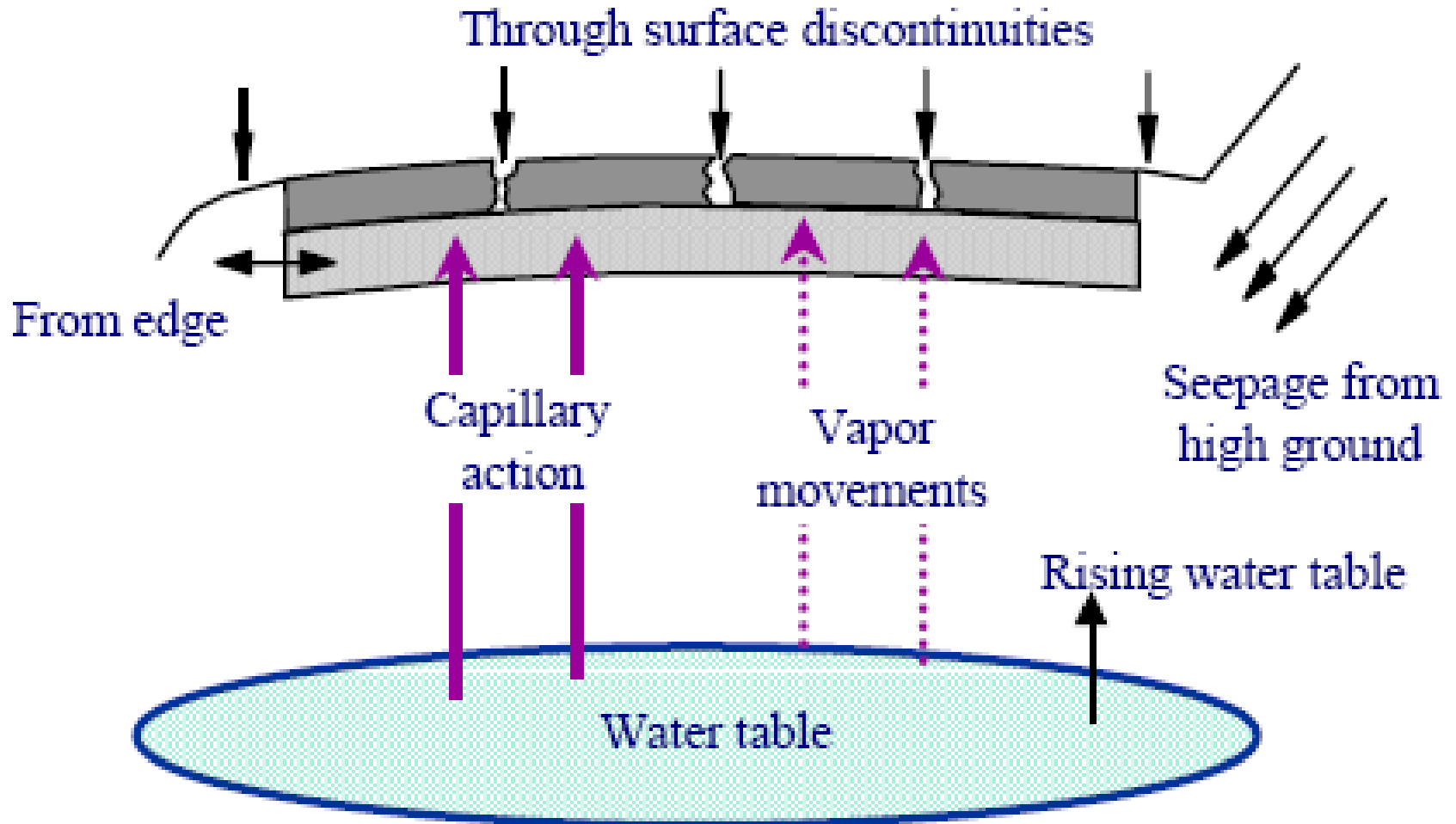
## IDENTIFY GOALS TO ENSURE AN ADEQUATE FOUNDATION

- Reduce soil PI and clay/silt-sized particles
- Improve workability and constructability
- **Reduce shrink/swell characteristics of expansive soils**
- **Improve strength and stiffness/modulus**
- Improve stability and durability
- **Reduce moisture susceptibility**

# EXPANSIVE SOIL AND EXPANSIVE CLAY



# MOISTURE IS THE ENEMY OF SUCCESSFUL PAVEMENT PERFORMANCE



# Problems Associated w/Clay Soils

- **Typically moisture sensitive**
  - **expansion potential & swell pressure**
  
- **Exhibit poor pavement support**
  - **low R-values, CBRs, & unconfined compressive strengths**
  
- **Constructability problems**
  - **highly plastic - poor workability**
  - **hard to compact**
  - **yield or pump when wet**
  - **shrink when dry**

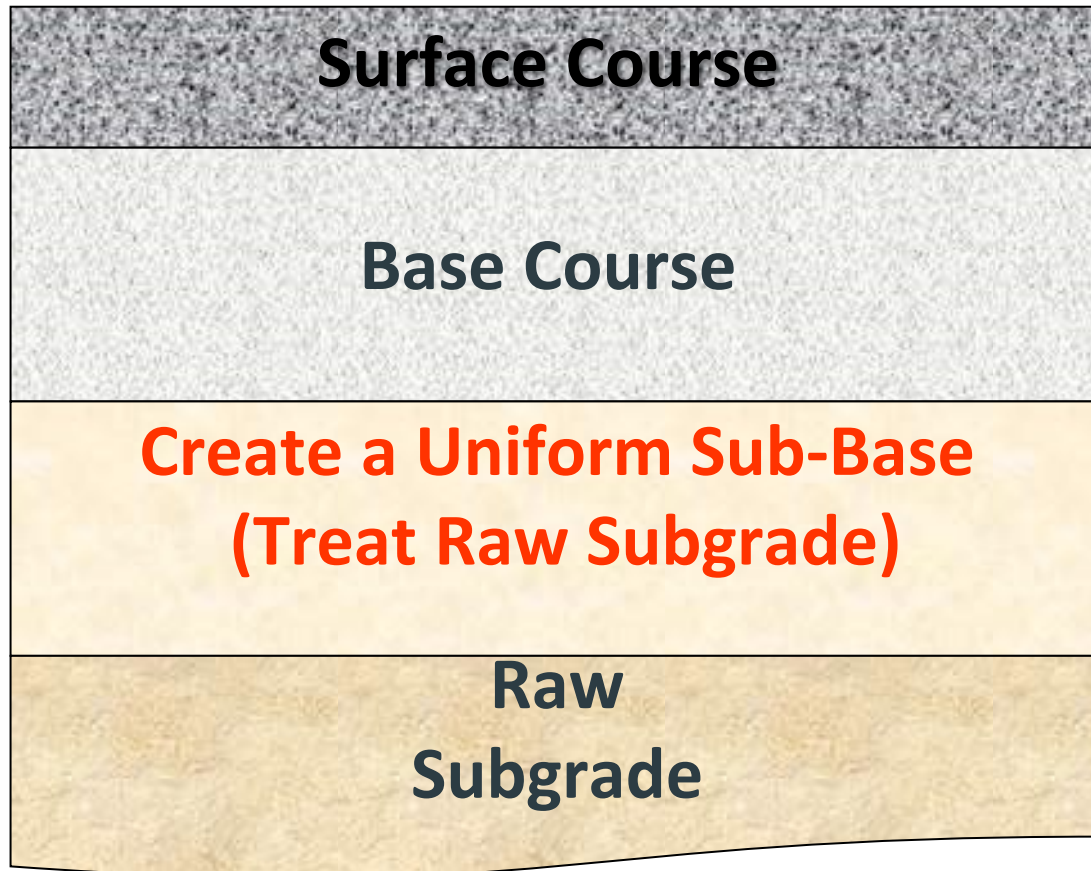


## FOUNDATION PROBLEMS – SOIL MOISTURE FLUCTUATION





# Need Uniform, Quality Layers Beneath HMA Surface



Moduli = 10 ksi

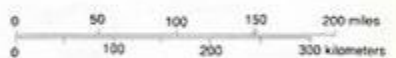
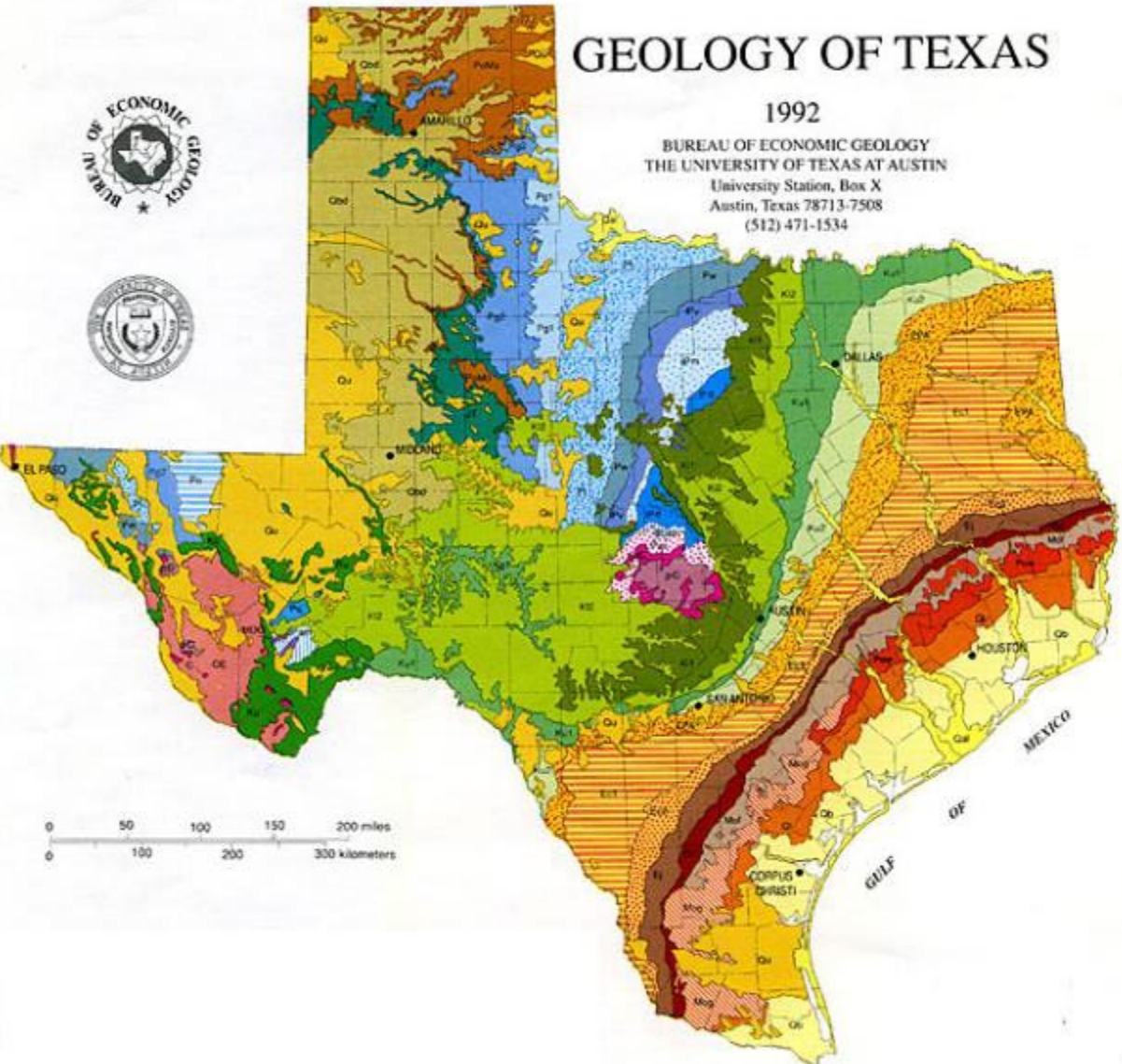
## BASE MATERIALS

- Provide structure and support for asphalt layer
- Bases can be untreated, utilizing good source material
- Marginal quality materials used for bases
  - Can be treated w/ stabilizers if they are marginal in quality
- **Improve strength and stiffness/modulus**
- Improve stability and durability
- **Reduce moisture susceptibility**

# GEOLOGY OF TEXAS

1992

BUREAU OF ECONOMIC GEOLOGY  
THE UNIVERSITY OF TEXAS AT AUSTIN  
University Station, Box X  
Austin, Texas 78713-7508  
(512) 471-1534



## EXPLANATION

| CENOZOIC  | Time (m.y.)   | Formation/Group   | Color/Pattern                         |              |   |            |
|---|---|---|---------------------------------------|--------------|---|------------|
|   |   |   |                                       | Quaternary   |   |            |
| CENOZOIC  | Quaternary  | Alluvium (Qal)  | Yellow                                |              |   |            |
|   |   | Quaternary undivided (Qu)                                 | Light yellow                          |              |   |            |
|   | Tertiary  | 2 m.y.  | Beaumont Formation (Qb)               | Orange       |   |            |
|   |   | Pliocene (5 m.y.)   | Lissie Formation (Ql)                 | Light orange |   |            |
|   |   |   | Blackwater Draw Formation (Qbd)       | Dark orange  |   |            |
|   |   | Miocene (24 m.y.)   | Wills Formation (Pww)                 | Red          |   |            |
|   |   |   | Ogallala Formation (Pomo)             | Light brown  |   |            |
|   |   | Oligocene (30 m.y.)                                       | Goliad Formation (Pog)                | Dark brown   |   |            |
|   |   |   | Flaming and Oakville Formations (Mof) | Light green  |   |            |
|   |   | Eocene (58 m.y.)  | Catahoula Formation (Oc)              | Dark red     |   |            |
| Oligocene and Eocene undivided (OE) (volcanic rocks and conglomerates in Trans-Pecos Texas) | Light red   |   |                                       |              |   |            |
| Jackson Group (Whitsett, Manning, Wellborn, Caddell, Yazoo, and Moody's Branch Fms.) (Ej)   | Dark brown  |   |                                       |              |   |            |
| Clabornes Group (Yegua Formation) (Ec2)   | Light green   |   |                                       |              |   |            |
| Paleocene (66 m.y.)   | Clabornes Group (Cook Mountain, Sparta, Weches, Queen City, and Reklaw) (Ec1) | Light green   |                                       |              |   |            |
|   | Wilcox and Midway Groups (EPA)  | Light green   |                                       |              |   |            |
| MESOZOIC  | Dietaceous (144 m.y.)   | Navarro and Taylor Groups (Ku2)                           | Light green                           |              |   |            |
|   |   | Austin, Eagle Ford, Woodbine, and U. Washita Groups (Ku1) | Light green                           |              |   |            |
|   | Fredericksburg and L. Washita Groups (Kl2)                                    | Light green   |                                       |              |   |            |
|   | Trinity Group (K01)   | Light green   |                                       |              |   |            |
|   | Cretaceous undivided (Ku)   | Light green   |                                       |              |   |            |
| Jurassic Triassic undivided (JT)  | Light green   |   |                                       |              |   |            |
| PALEOZOIC   | Time (m.y.)   | Formation/Group   | Color/Pattern                         |              |   |            |
|   |   |   |                                       | 245 m.y.     | Ochoan Series (Po)  | Light blue |
|   |   |   |                                       | 286 m.y.     | Guadalupian Series (Whitehorse and Quaternaster Formations) (Pg2) | Light blue |
|   |   |   |                                       |              | Guadalupian Series (Blaine and San Angelo Formations) (Pg1)       | Light blue |
|   |   |   |                                       | 320 m.y.     | Leonardian Series (Pl)  | Light blue |
|   |   |   |                                       |              | Wolfcampian Series (Pw)   | Light blue |
|   |   |   |                                       | 505 m.y.     | Permian undivided (Pu)  | Light blue |
|   |   |   |                                       |              | Virgilian Series (IPv)  | Light blue |
|   |   |   |                                       | 570 m.y.     | Missourian Series (IPm)   | Light blue |
|   |   |   |                                       |              | Desmoinesian Series (IPd)   | Light blue |
| 1200 m.y.   | Altkan and Monrovia Series (IPam)   | Light blue  |                                       |              |   |            |
|   | Mississippian, Devonian, and Ordovician undivided (MDO)                       | Light blue  |                                       |              |   |            |
| Pre-cambrian  | 570 m.y.  | Cambrian (C)  | Light blue                            |              |   |            |
|   | 2000 m.y.   | Paleozoic undivided (Pau)                                 | Light blue                            |              |   |            |
| Pre-cambrian  | 2000 m.y.   | Precambrian undivided (p-C)                               | Light blue                            |              |   |            |

# INDUSTRIAL MINERALS OF TEXAS

2008

BUREAU OF ECONOMIC GEOLOGY

SCOTT W. FINKER, DIRECTOR  
JOHN A. AND KATHERINE G. JACKSON SCHOOL OF GEOSCIENCES  
THE UNIVERSITY OF TEXAS AT AUSTIN

University Station, Box X  
Austin, Texas 78713-8924  
(512) 471-1534



This publication was co-funded by the Jackson School of Geosciences.

Map and accompanying text compiled by J. Richard Kyle.

Map and text reviewed by U.S. Geological Survey Minerals Information Team.



### MAJOR PRODUCING AREAS

#### Construction Materials

- C-1 Cement plant
- C-2 Common clay
- C-3 Crushed stone
- D-1 Dimension granite
- D-2 Dimension limestone
- D-3 Dimension sandstone
- G-1 Gypsum
- SG Construction sand and gravel

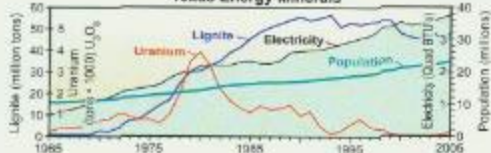
#### Chemical Materials

- B-C Ball clay and kaolin
- B-1 Bentonite
- H-1 Helium
- I-S Industrial sand
- L-1 Lime plant
- S-1g Sulfur (natural gas)
- S-1 Sulfur (oil)
- S-2 Salt
- SS Sodium sulfate
- T-1 Talc
- Z-1 Zeolites
- Concentration of mineral operations
- Salt dome

#### Energy Minerals

- Bituminous coal
- Lignite coal
- U Uranium
- Mouth-of-river electricity plant

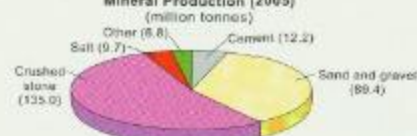
### Texas Energy Minerals



| Million years ago |            | Dominant Rock Types |   |
|-------------------|------------|---------------------|---|
| CENOZOIC          | Quaternary | 2                   | Unconsolidated sands and muds   |
|                   | Tertiary   | 65                  | Sandstones and mudstones (volcanics in Trans-Pecos)                             |
| MESOZOIC          | Upper      | 145                 | Limestones (sandstones and mudstones in Trans-Pecos Texas)                      |
|                   | Lower      | 245                 | Sandstones and mudstones  |
| PALEOZOIC         | Upper      | 320                 | Pennsylvanian carbonates and evaporites; Mississippian sandstones and mudstones |
|                   | Lower      | 540                 | Limestones, dolomites, chert, shale sandstones                                  |
| PRECAMBRIAN       |            | 540                 | Granite intrusions and metamorphic rocks  |

Unconformity: A boundary between two rock units that represents a gap in the geologic record due to erosion or nondeposition of rock.

### Mineral Production (2005)



### Mineral Value (2005)



\*Other\* includes brych, clays (bar), bentonite, common, fuller's earth, kaolin, dimension stone, gypsum, helium, talc, and zeolites.



# Flexible Base Selection and Information Guide

---

Materials & Tests Division

Soils & Aggregates Section

August 2019

*Table 1: Flexible Base Material Types*

| Type     | Description   |
|----------|---|
| <b>A</b> | Crushed stone produced and graded from oversize quarried aggregate that originates from a single, naturally occurring source. This does not include gravel or multiple sources.   |
| <b>B</b> | Crushed or uncrushed gravel. Blending of two or more sources is allowed.  |
| <b>C</b> | Crushed gravel with a minimum of 60% of the particles retained on a No. 4 sieve with two or more crushed faces as determined by Tex-460-A, Part I. Blending of two or more sources is allowed.  |
| <b>D</b> | Type A material or crushed concrete. Crushed concrete containing gravel will be considered Type D material. Crushed concrete must meet requirements for recycled materials and be managed in a way to provide for uniform quality. The engineer may require separate dedicated stockpiles to verify compliance. |
| <b>E</b> | Caliche, iron ore, or as otherwise shown on the plans.  |

*Table 2: Basic Recommendations for Type Selection*

| Type    | Description                  |
|---------|------------------------------|
| A or D* | Strongest, most durable base |
| B or C  | Marginal base                |
| E       | Non-standard requirements    |

\* When crushed concrete is allowed.

Type A and D materials are generally considered high-quality base since crushed materials have, in general, higher stability than rounded materials. Type A and D are often used in combination with Grade 1–2, which has the most stringent material requirements. Type D allows the use of Type A or crushed concrete. This option provides an alternative where crushed concrete may be used if economically feasible.

Types B and C are generally used for areas that have gravel as a local material.

Type E may be used for new or unspecified materials.

*Table 3: Flexible Base Material Requirements from Item 247*

| Property  | Test Method | Grade 1-2             | Grade 3 | Grade 4               | Grade 5 |
|---|-------------|-----------------------|---------|-----------------------|---------|
| Master gradation sieve size (cumulative % retained) | Tex-110-E   |                       |         | As shown on the plans |         |
| 2½"   |             | 0                     | 0       |                       | 0       |
| 1¾"   |             | 0-10                  | 0-10    |                       | 0-5     |
| 7/8"  |             | 10-35                 | -       |                       | 10-35   |
| ¾"  |             | 30-65                 | -       |                       | 35-65   |
| #4  |             | 45-75                 | 45-75   |                       | 45-75   |
| #40   |             | 65-90                 | 50-85   |                       | 70-90   |
| Liquid limit, % max <sup>1</sup>                    | Tex-104-E   | 40                    | 40      | As shown on the plans | 35      |
| Plasticity index, max <sup>1</sup>                  | Tex-106-E   | 10                    | 12      | As shown on the plans | 10      |
| Plasticity index, min <sup>1</sup>                  | Tex-106-E   | As shown on the plans |         |                       |         |
| Wet ball mill, % max <sup>2</sup>                   | Tex-116-E   | 40                    | -       | As shown on the plans | 40      |
| Wet ball mill, % max increase passing the #40 sieve | Tex-116-E   | 20                    | -       | As shown on the plans | 20      |
| Compressive strength, psi, min                      | Tex-117-E   |                       |         | As shown on the plans |         |
| Lateral pressure, 0 psi                             |             | 35                    | -       |                       | -       |
| Lateral pressure, 3 psi                             |             | -                     | -       |                       | 90      |
| Lateral pressure, 15 psi                            |             | 175                   | -       |                       | 175     |

<sup>1</sup> Determine plastic index in accordance with Tex-107-E (linear shrinkage) when liquid limit is unattainable, as defined in Tex-104-E.

<sup>2</sup> Grade 4 may be further designated as Grade 4A, Grade 4B, etc.



# FULL DEPTH RECLAMATION (FDR)



3088

---

## Special Specification 3088

# Full Depth Reclamation Using Foamed Asphalt (Road-Mixed)



---

### 1. DESCRIPTION

Perform full depth reclamation (FDR) using an in-place mixing process to obtain a homogenous mixture of the existing surface and the underlying base material (with or without new material and additive added) using a foamed asphalt.

3089

---

## Special Specification 3089

### Full Depth Reclamation Using Asphalt Emulsion (Road-Mixed)



---

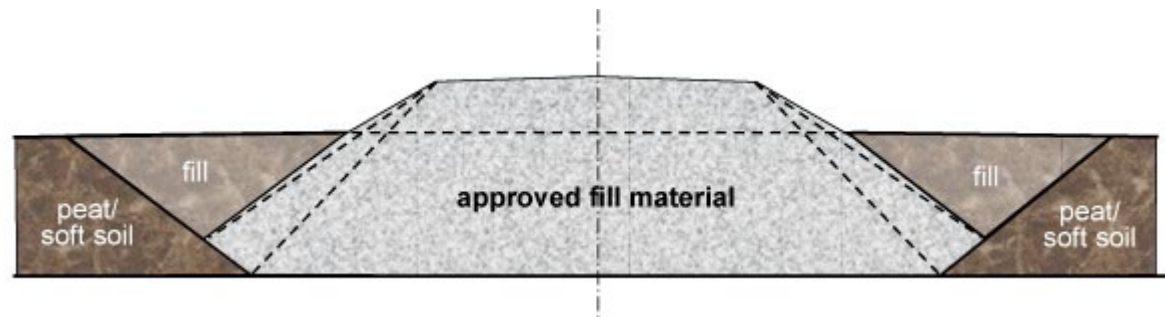
#### 1. DESCRIPTION

Perform full depth reclamation (FDR) using an in-place mixing process to obtain a homogenous mixture of the existing surface and the underlying base material (with or without new material and additive added) using an emulsified asphalt.

## COMMON METHODS USED TO ADDRESS PROBLEM BASES AND SOILS

- Lime
- Cement
- Asphalt emulsion / foamed
- Remove and Replace Existing Subgrade / Embankment
- Add Additional Base or Pavement Thickness Instead of Treating the Subgrade
- Geo-Grid Products
- Alternative Stabilizers (Proprietary Liquids or Dry)
- Recycling Existing Pavement and Base (FDR)

# REMOVE POOR SOILS AND IMPORT SELECT FILL OR BASE MATERIAL



## REMOVE POOR SOILS AND IMPORT SELECT FILL OR BASE MATERIAL (CUT AND FILL)

- This is almost never cost effective compared to lime treatment
- Removal and replace costs alone are typically > than \$10/CY
- Highly variable determining cost of import material due to source and haul distance
- **This is often a strategy when sulfates are present in the clay**
- Can be other problems associated with this concept that aren't considered (variability of import fill source that isn't properly monitored)

## REMOVE POOR SOILS AND IMPORT SELECT FILL OR BASE MATERIAL (CUT AND FILL)

- For 8" Lime:
  - 1800 tons (\$275/ton) = \$495,000
  - 100,000 SY @\$4.00 = \$400,000
  - Total = \$895,000**
- Removal and replace costs alone are > than \$10/CY
  - 100K SY @ 2' deep = 67K CY
  - 67,000 CY @\$14.00 = \$938,000
    - Disposal @\$5.00 = \$335,000
    - Select Fill @\$5.00 = \$335,000
  - Total = \$1,608,000**

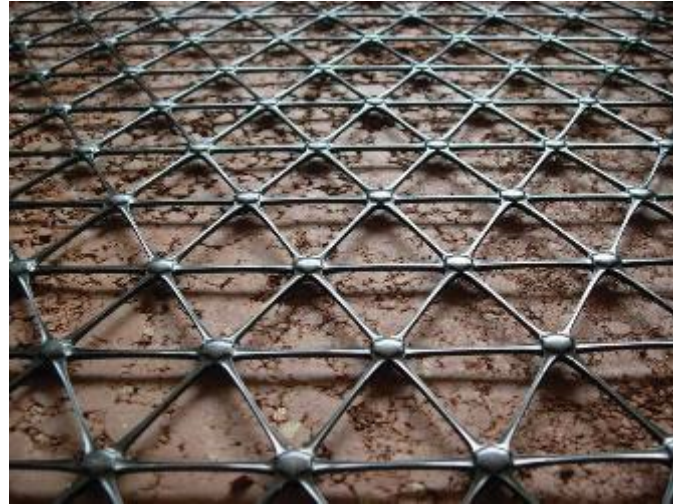
**\*The above case is an example and varies based on fill sources and costs**

## INCREASING THICKNESS OF OTHER LAYERS IN THE PAVEMENT SECTION

- This alternative appears most commonly in commercial and subdivision type projects
- The engineer/architect or sometimes a contractor will convince a developer or a smaller local agency to do away with lime treatment by adding an extra inch of concrete pavement, or extra flex base and sometimes Hot Mix Asphalt (HMA)
- This is a strategy that is driven purely by cost and time
- This will, in almost all cases, lead to poor performance for roadways, parking lots, and foundations because nothing has been done to treat the underlying problems of highly plastic swelling clays



# THE USE OF GEOGRID IN PLACE OF SOIL STABILIZATION



## THE USE OF GEOGRID IN PLACE OF SOIL STABILIZATION

- Geogrids are Polypropylene products that are being used in a variety of ways
- Geogrid is a very good technology, when used in the proper situation
- They base their value add on improving tensile strength in the pavement base layer. Coupled with cost savings and speed of construction

## THE USE OF GEOGRID IN PLACE OF SOIL STABILIZATION

- Using Geogrid directly on compacted but untreated high PI subgrade will not provide the benefits achieved by creating a uniform lime stabilized layer.
  - Moisture and the fines in the untreated subgrade layer will work their way up into the crushed stone base (fluctuations in moisture condition and capillary rise). This weakens the base layer significantly over time and can lead to premature failure of the pavement structure.
  - A properly treated sub-base layer provides significant protection for the overlying base layer by adding strength, waterproofing and stopping the swell potential of the treated layer that will not happen using Geogrid alone
  - The use of GeoGrid on top of a properly stabilized subgrade is a very good option

## USE OF PROPRIETARY LIQUID OR DRY STABILIZATION PRODUCTS

- (1.) ionic stabilizers, reported to work through cation exchange within the clay mineral
- (2.) enzyme stabilizers, described as consisting of various organic catalysts
- (3.) polymer and “Bio-Polymer” stabilizers, comprised of various organic and inorganic polymers

## USE OF PROPRIETARY LIQUID OR DRY STABILIZATION PRODUCTS

*“Supplier claims of product effectiveness are often not well substantiated with independent field or laboratory evaluations performed under controlled conditions. The chemical composition of these products is usually considered proprietary, and the suppliers often give minimal or incomprehensible information regarding the mechanisms of soil modification.”*

---

AN ANALYSIS OF THE MECHANISMS AND EFFICACY OF THREE LIQUID CHEMICAL SOIL STABILIZERS: VOLUME 1

May 2003

Alan F. Rauch, Lynn E. Katz, and Howard M. Liljestrand

## What is Soil Treatment ?

- **Soil Treatment** : general term to indicate a process aimed at physically modifying a soil so that it can fulfill the purpose intended. (i.e. – moisture conditioning, scarify & re-compact, soil modification, soil stabilization & etc.).
- **Soil Modification** : operation which improves the engineering properties of a soil by addition of lime. The additive is added in a quantity insufficient to induce a significant permanent chemical change. Improves texture and creates stable working platform.

# What is Soil Treatment ?

## Continued

- **Soil Stabilization** : operation which permanently alters the chemical and physical characteristics of the soil in a way that renders it stable, particularly with respect to the action of water and frost. It gives a permanent strength that can be measured by typical methods such that target moduli is achieved.

# Soil Treatment

(know your goals):



**Very wet jobsite**

## **Soil Modification/ Dry Up:**

Useful to expedite construction or to create a stable working platform

May not result in permanent changes to soil

(intentional i.e. – temporary haul road or storage yard & not intentional i.e. – over mixing of depth & inconstant application rate resulting in dilution)



# Soil Treatment

(know your goals):



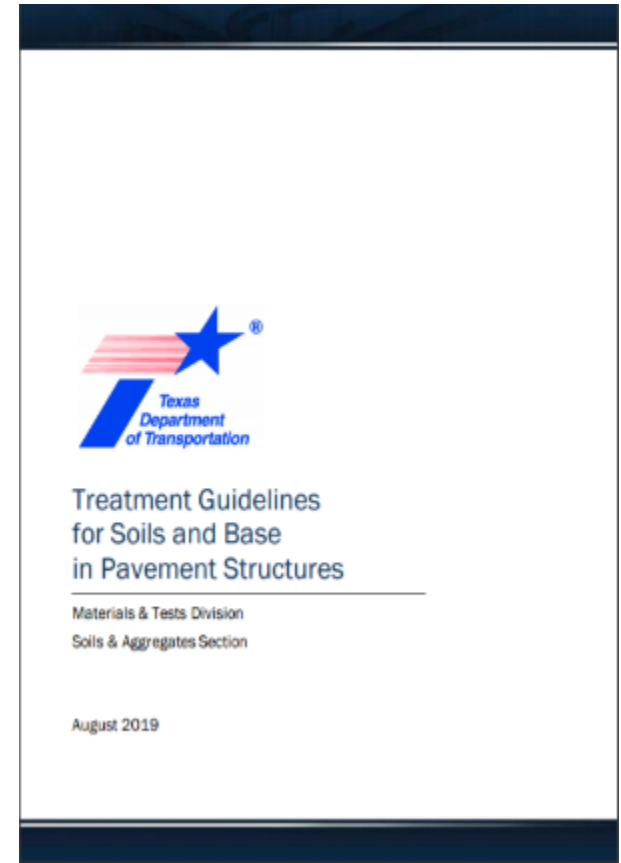
## **Soil Stabilization:**

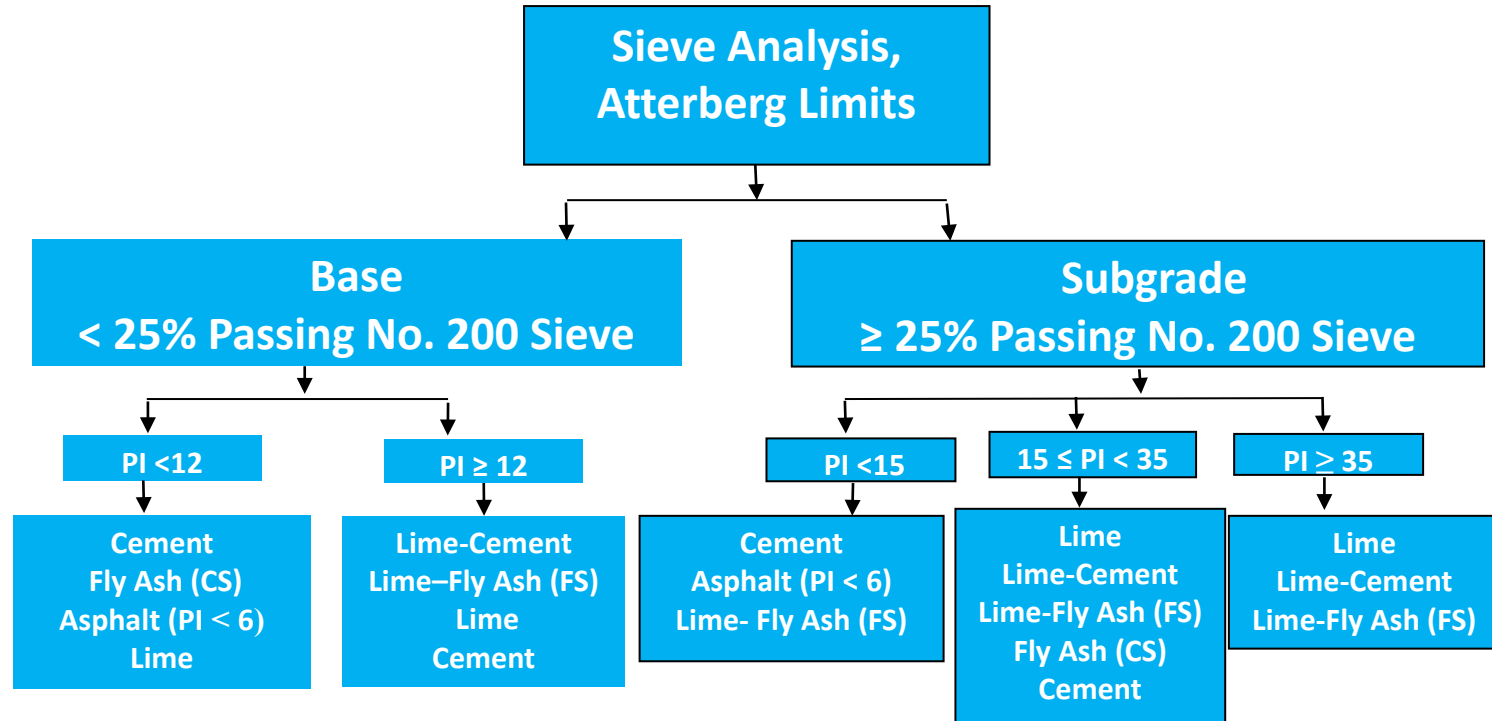
engineered system  
designed to  
maximize structural  
strength and  
permanently alter  
soil system

## PURPOSE OF THE TREATMENT GUIDELINES

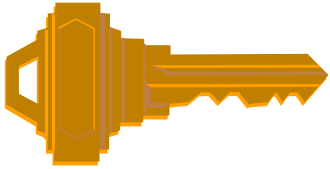
<http://ftp.dot.state.tx.us/pub/txdot/mtd/treatment-guidelines.pdf>

- Provide personnel with all levels of experience with enough information to
- - Determine project suitability,
  - Identify goals of treatment,
  - Select appropriate type of additives,
  - Determine optimum amount of selected additives, and
  - Identify appropriate construction processes.
- The information in this document is applicable to both construction and maintenance.










| TxDOT Specs (Road or Plant Mixed) |                  | TxDOT Lab Mix Design Test Methods |
|-----------------------------------|------------------|-----------------------------------|
| Lime                              | Items 260 or 263 | Tex-121-E                         |
| Cement                            | Items 275 or 276 | Tex-120-E                         |
| Fly Ash                           | Item 265         | Tex-127-E                         |
| Asphalt                           | Item 292         | Tex-126-E                         |



## KEY POINTS OF GUIDELINES

-  Soils are different
-  Additives are different
-  Reactions between soils and additives are different
-  **DO A MIX DESIGN**
-  Evaluate the material engineering properties and field performance

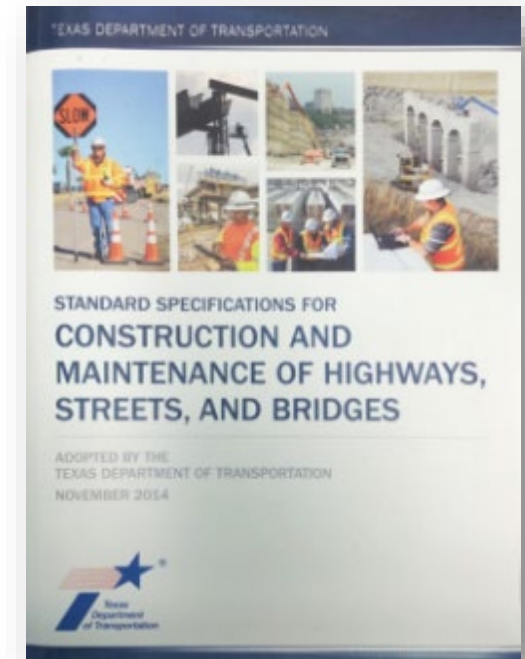


## ➤ Select Additives

- Select the appropriate type of additive based on the goals of soil treatment and the project-specific soil classification (gradation and PI)
- Perform the lab mix designs to determine the optimum amount of the selected additive on a project basis for the desired engineering properties
  - Soil-Lime (Tex-121-E); DMS 6350
  - Soil-Cement (Tex-120-E); DMS 4600
  - Lime-Fly Ash (Tex-127-E); DMS 4615
  - Asphalt Treated Base (Tex-126-E)

# CONSTRUCTION SPECIFICATIONS

- **A Perfect Laboratory Mix Design Will Only Work in the Field if Proper Construction Procedures Are Followed**



## TXDOT SOIL TREATMENT CONSTRUCTION SPECIFICATIONS

| Item             | Treatment                            | Special Requirements   |
|------------------|--------------------------------------|--|
| 260              | Lime (road mixed)                    | Mellowing requirements apply depending on the type of lime and sulfate content of the material.  |
| 263              | Lime (plant mixed)                   | Cure at least 7 days by sprinkling or by asphalt membrane.   |
| 265              | Fly ash or lime-fly ash (road mixed) | Complete compaction within 6 hours of application of Class FS ash and within 2 hours of application of Class CS ash. Cure FS ash 7 days; CS at least 24 hours. |
| 275              | Cement (road mixed)                  | Complete compaction within 2 hours. Microcrack to reduce shrinkage cracks. Cure at least 3 days by sprinkling or asphalt membrane.                             |
| 276              | Cement (plant mixed)                 | Microcrack to reduce shrinkage cracks. Cure at least 3 days by sprinkling or asphalt membrane.   |
| Asphalt Emulsion | Emulsion (road mixed)                | Cure to 2 percentage points below optimum before placing the next course.  |
| Foamed Asphalt   | Foamed Asphalt (road mixed)          | Special equipment is needed. Cure a minimum of 2 hours.  |

# Lime Stabilization is Not to Hard to Achieve



## Do These Things

- Apply right amount of lime
- Control mix depth
- Use plenty of water

**Hard to mess up!**



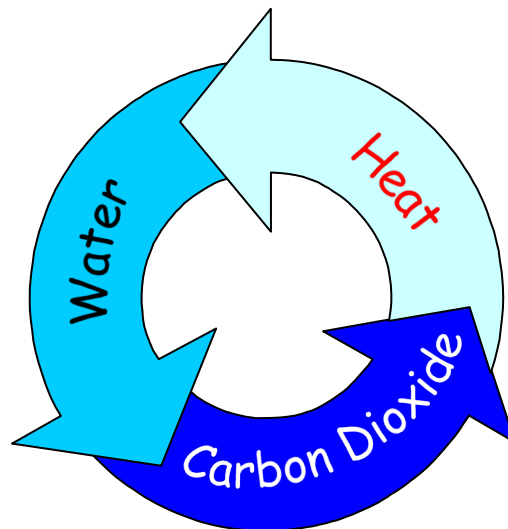
# The Lime Cycle



**CaO**  
**Quicklime**



**Ca(OH)<sub>2</sub>**  
**Hydrated Lime**



**CaCO<sub>3</sub>**  
**Limestone**

# Manufacture of Lime



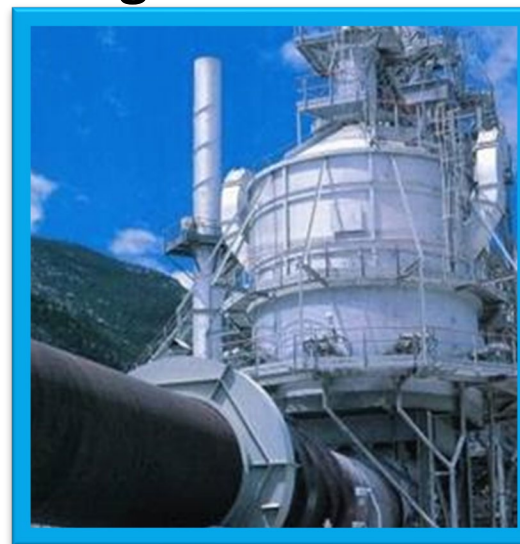
**Lime Reserves**



**Blasting**



**Mining Operations**



**Rotary Kiln w/Preheater**

## Manufacture of Lime

- To transform  $\text{CaCO}_3$  to  $\text{CaO}$  takes temperatures  $> 1800$  °F.
- Most US kilns are rotary designs, up to 400 feet long and 16 feet in diameter producing 100 - 1,300 tpd
- Vertical Kilns (Mertz) are also used and are very efficient

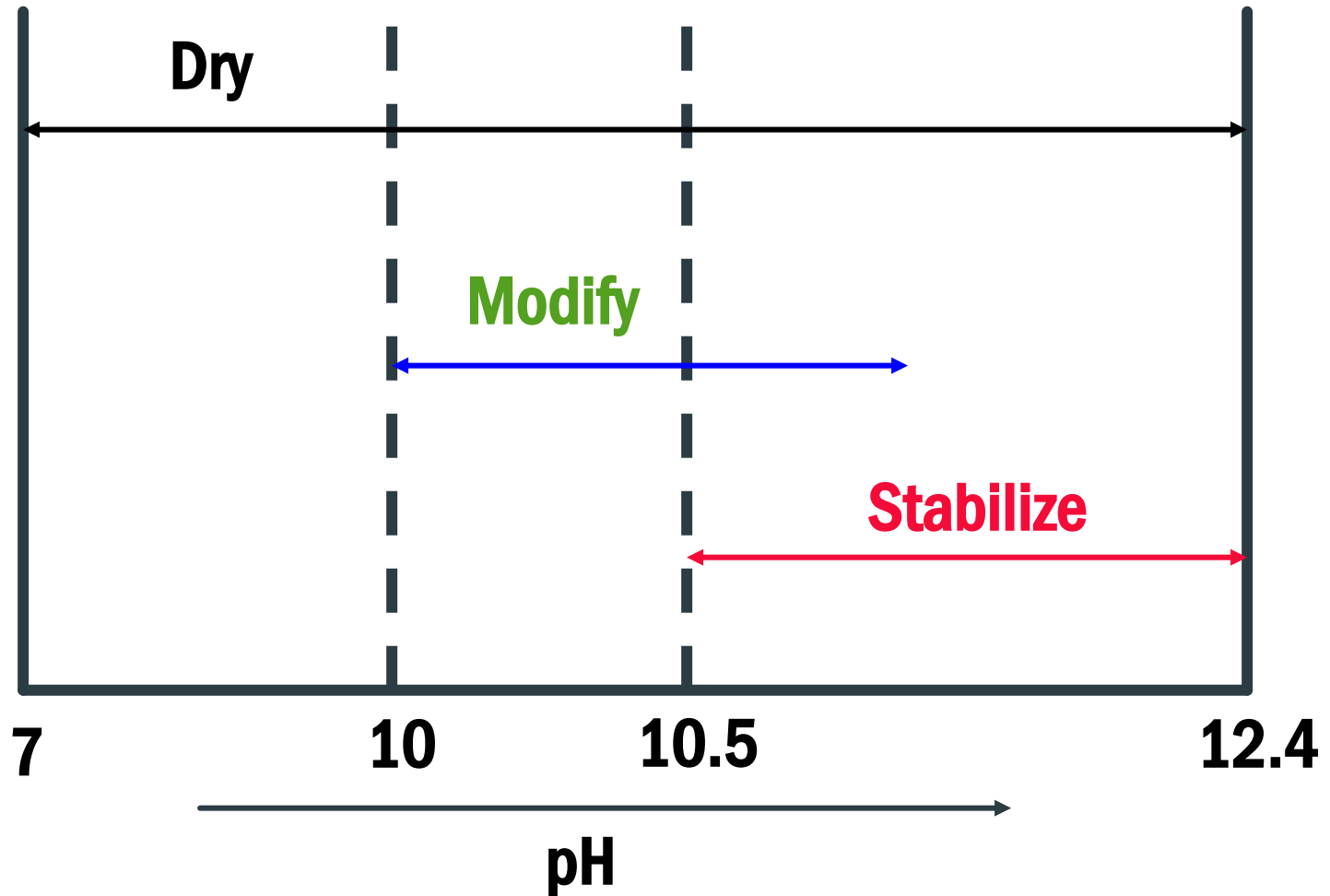


## Manufacture of Lime

- Lhoist also operates Vertical Kilns and has commissioned 3 of these new kilns in the U.S. in the last 2 years. They are more efficient



# USES OF LIME



## Benefits of Lime Stabilization

- The addition of lime can permanently change the clay soils, stabilizing and strengthening them into high performance pavement foundations
- Lime permanently reduces the plasticity of expansive soils
- A properly stabilized layer improves the uniformity of the subgrade (layer beneath base)

# Structure of Clay Minerals



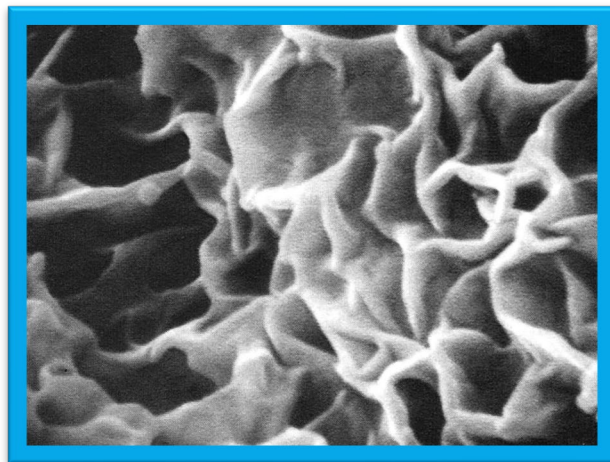
## **Kaolinite:**

Hexagonal crystals

Size: 0.2 - 2  $\mu\text{m}$

Surface Area: 10 - 30  $\text{m}^2/\text{g}$

[Magn: 2000 x]



## **Montmorillonite:**

Flakes

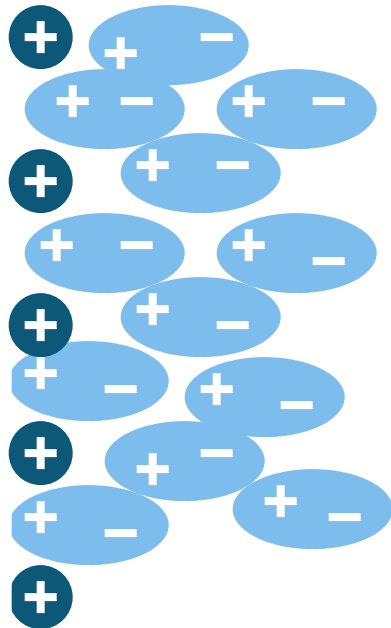
Size: 0.01 - 1  $\mu\text{m}$

Surface Area: 650 - 800  $\text{m}^2/\text{g}$

[Magn: 20000 x]

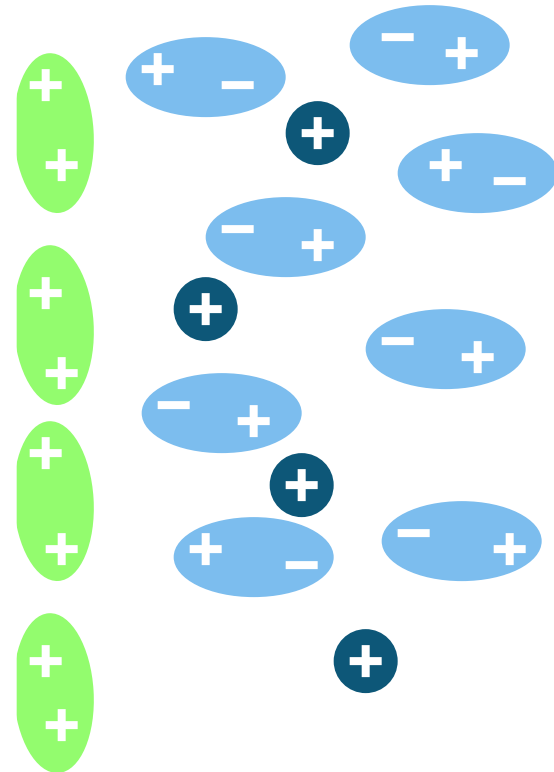
# THE SYSTEM: CLAY – WATER - CALCIUM

Clay Surface



Negatively charged clay surface attracts cations (+) & water molecules (dipole), causing formation of a 'double diffused water layer'

Clay Surface



Calcium cations (++) replace water and lessens ions so soil becomes more workable



# Lime - Soil Reactions (1):

## Immediate Reactions (within hours)

- **Reduction in Water Content**  
chemical reaction ( $\text{CaO} + \text{H}_2\text{O} \rightarrow \text{Ca}(\text{OH})_2 + \text{heat}$ )\* and mixing effect \*: not for Hydrated Lime
- **Flocculation / Agglomeration of Clay particles**  
textural change leads to decrease in PI & increase in workability

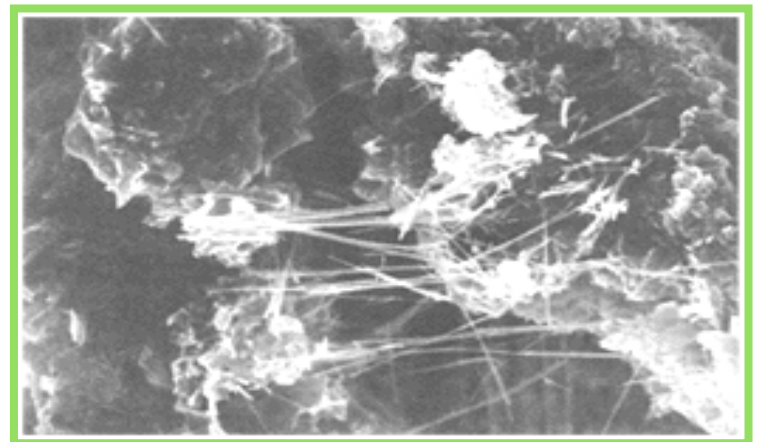


## Lime - Soil Reactions (2):

### Medium & Long Term Reaction (weeks, months, years)

#### ➤ Pozzolanic reaction between lime and clay particles

- Rate of pozzolanic reaction depends on
  - amount and type of clay mineral
  - temperature (increase by 50°F doubles speed of reaction)
  - high pH (> 12) and availability of calcium and water



# Pozzolan Reaction

## Pozzolan:

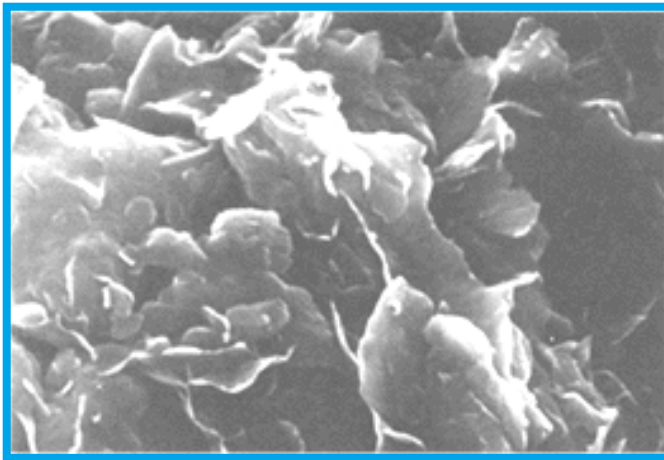
Siliceous or aluminous material that reacts in presence of water and Lime ( $\text{Ca}^{2+}$  &  $\text{OH}^-$ ) to produce stable, water-insoluble hydrates

- ▶ Calcium-Silicate-Hydrates (CSH)
- ▶ Calcium-Aluminate-Hydrates (CAH)

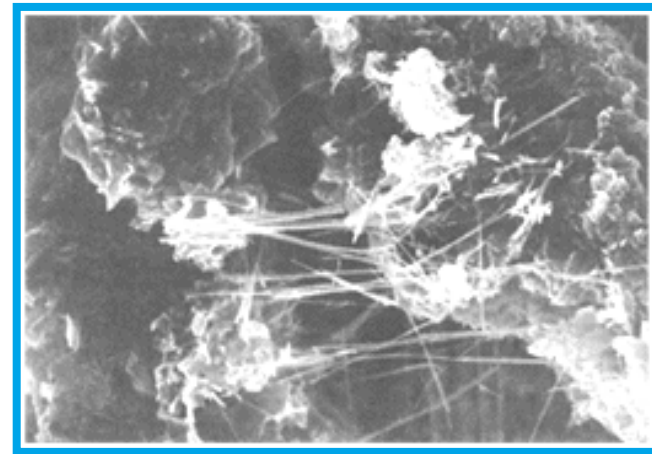
These products are essentially the same hydrates that form during the hydration of Portland cement.

# LIME STABILIZATION PROCESS

Hydrated lime ( $\text{Ca(OH)}_2$ )  $\longrightarrow$  high pH)  
+ water ( $\text{H}_2\text{O}$ )  
+ Clay (Silica & Alumina dissolve)  
= Cementitious material (CSH & CAH)

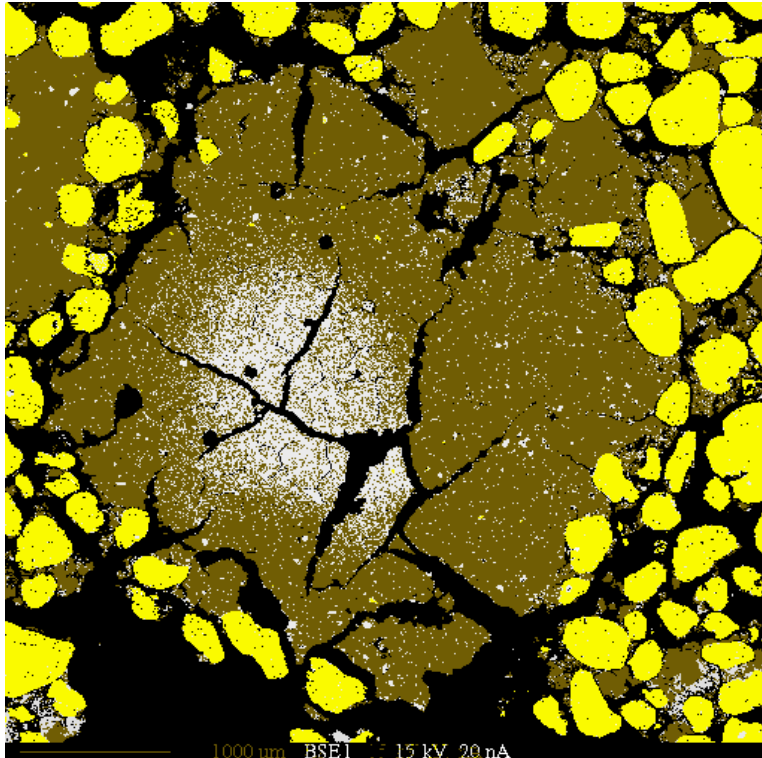


**Natural Clay**

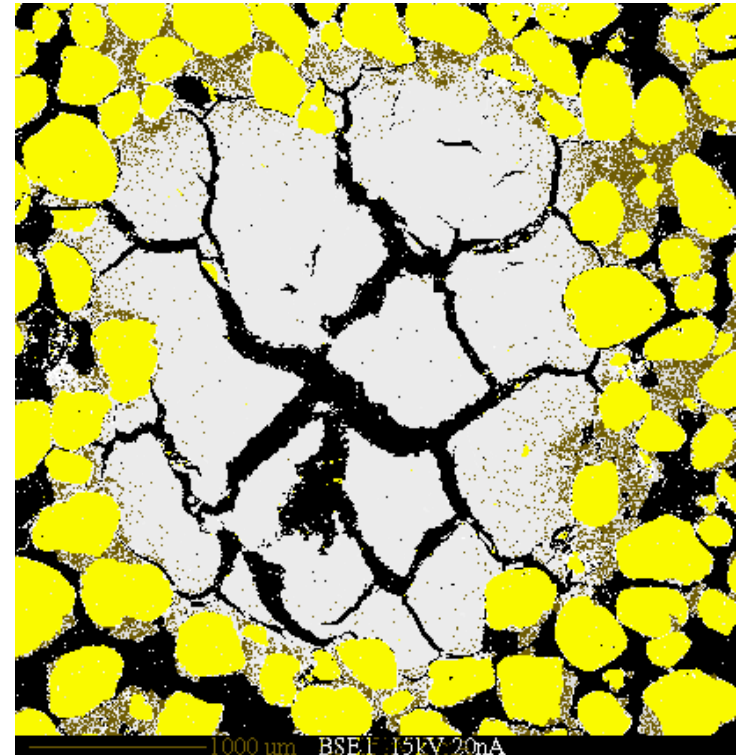


**Pozzolanic Reaction Increases  
Bonds by Formation of  
Crystals**

# CALCIUM DIFFUSION INTO CLAY – 365 DAYS



Lime treated, 5mm



Cement treated, 4mm

Ref: Harris, et al



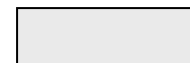
Si bearing



Porosity



Ca + Si + Al bearing



Si + Al bearing

# Tests for Lime Stabilization



**Tex-104, 105, & 106-E**



**Tex-121-E Part III**



**Tex-113-E**



**Tex-121-E Part I**

## SOIL – LIME TESTING:

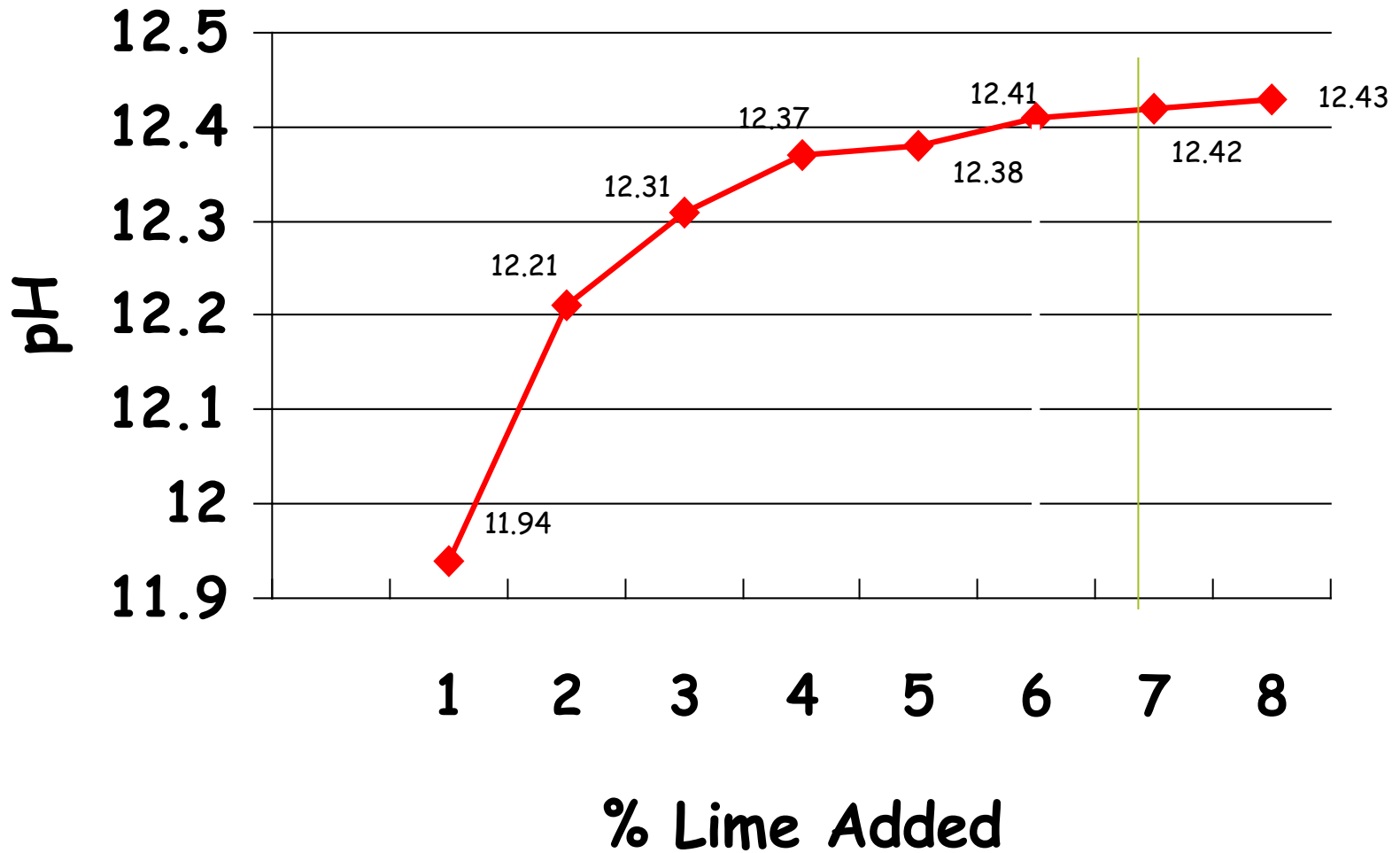
- **Determine Lime Demand**
  - Tex-121-E Part III (Eades & Grim pH test) to determine Lime demand
  
- **Determine MD Curve & Optimum Moisture Content**
  - Tex-113-E
  
- **Determine Strength**
  - Tex-121-E Part I to determine Unconfined Compressive Strength (UCS)

# TEX-121-E PART III



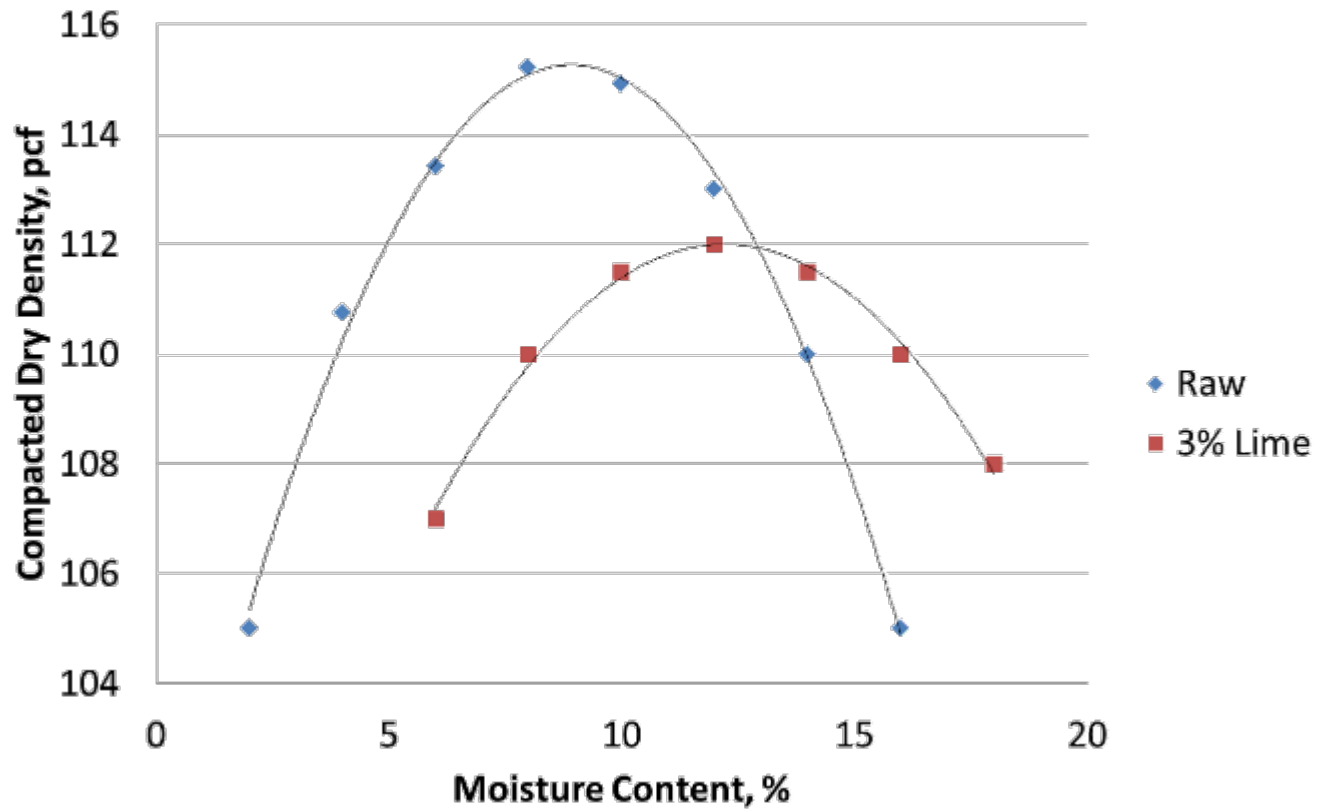


# Tex-121-E Part III



# Influence on Moisture-Density Relation:

**Moisture Density Relationship for Raw Soil and Lime Treated Soil**



## RAPID CHANGE IN SOIL TEXTURE



**Native Clay**

**Lime Treated Clay**

# BIG DIFFERENCE!

Before

After



# CHANGING IT FROM BAD TO GOOD

Before Lime Stabilization



After Lime Stabilization



# QUESTIONS?



# LIME STABILIZATION CONSTRUCTION



## Lime Soil Stabilization

- Lime spread rate
- Pulverization
- Mixing efficiency
- Lime content
- Depth of lime treatment
- Moisture content
- Density
- Slurry composition
- Weather limitations



# Lime Slurry



After Application



Jobsite Porta Batch

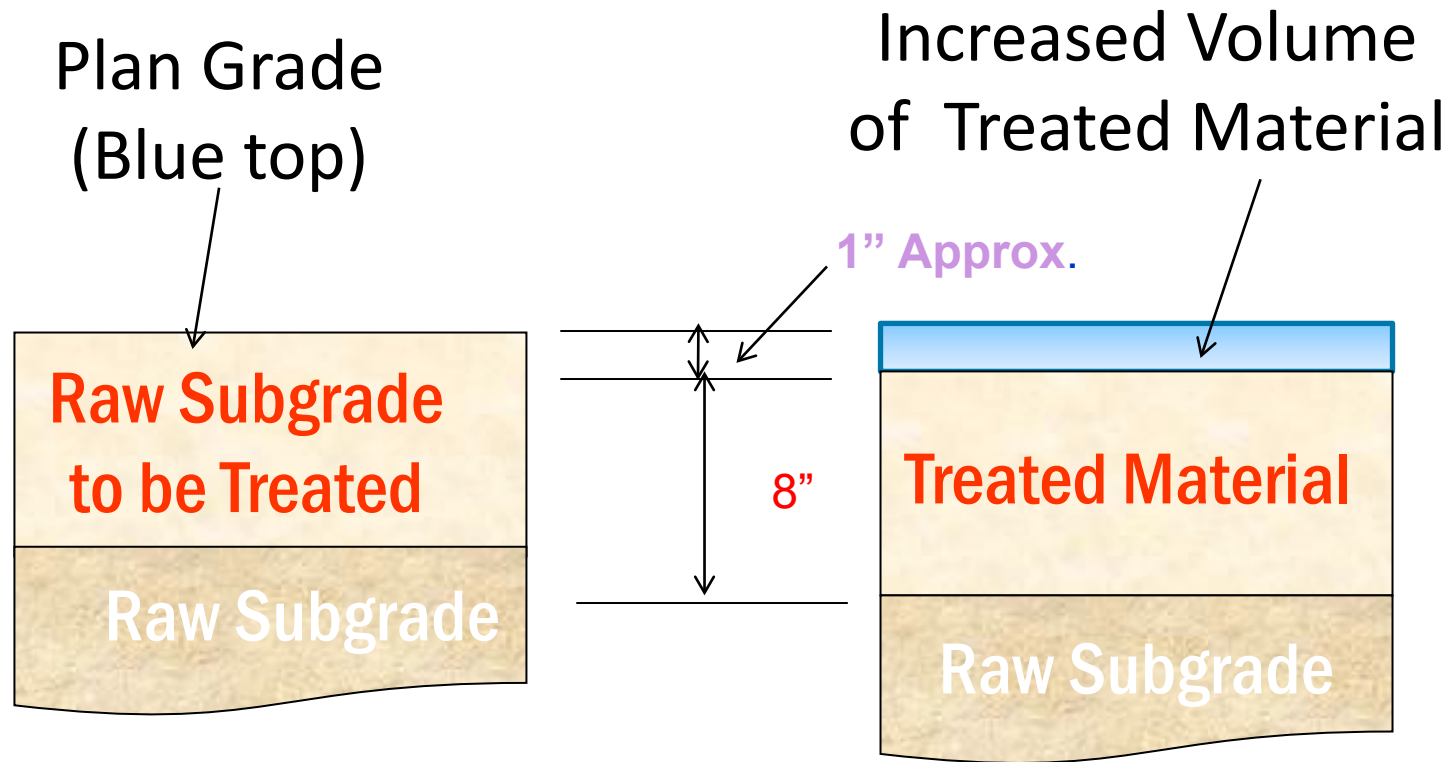
## SOIL PREPARATION

- Bring to final grade and alignment
- Adjust for fluff - minimize by using adequate water and mixing immediately
- Blade trim top 0.25-in. (BMP or Spec)



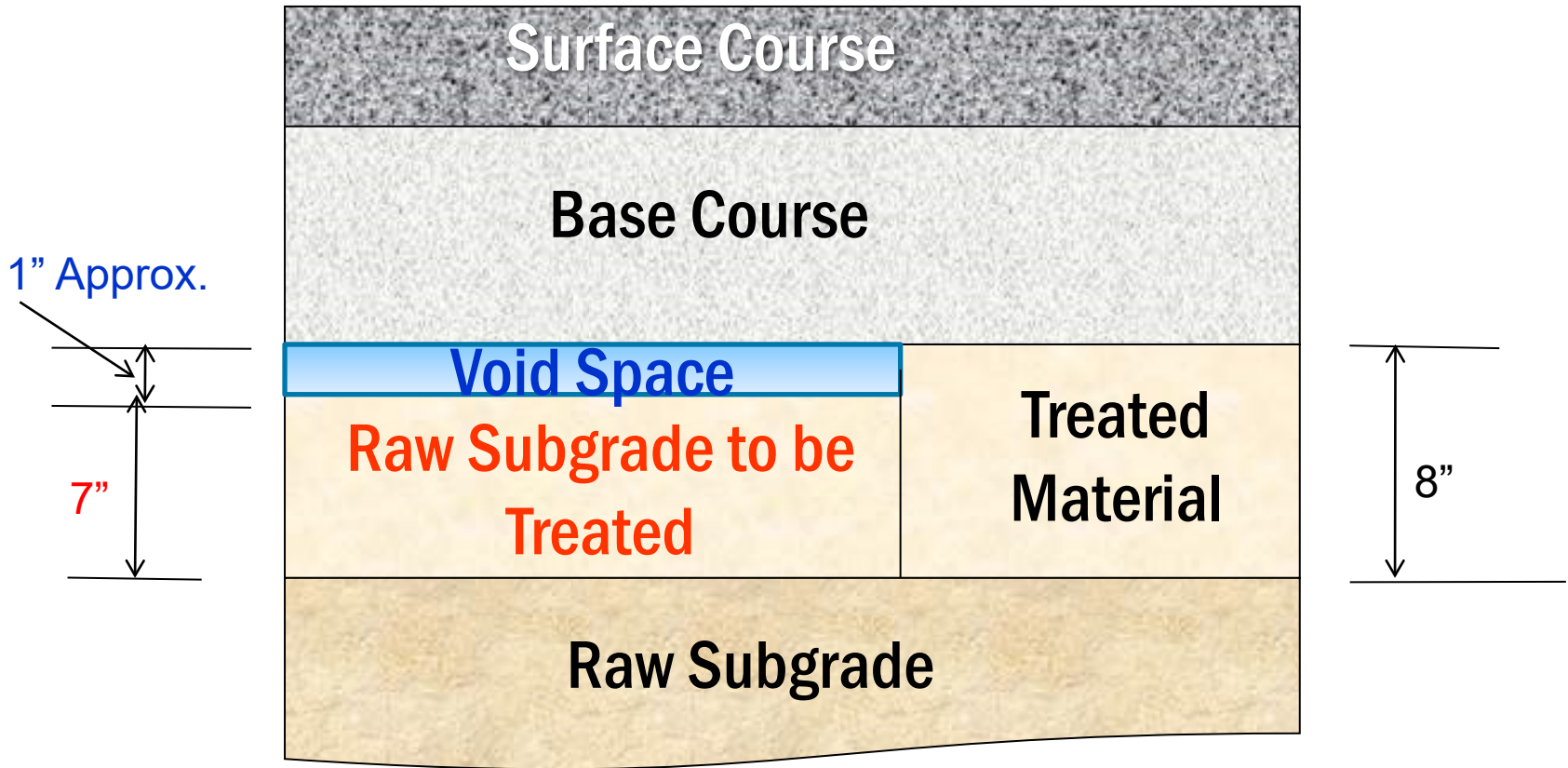
**In-situ subgrade before  
stabilization**

# Subgrade Adjustment for Fluff



Rule of Thumb: 1" of fluff for every 8" of treatment

# Subgrade Adjustment for Fluff



# LIME APPLICATION

- Dry lime - scarify to stabilization depth before lime is added (use BMP's or specs)
- Slurry - scarify prior to addition



## SPREADING OF LIME

➤ Apply only to an area where mixing can be completed on the same working day.

- Whether using dry lime or lime slurry it needs to be mixed into soil and additional water needs to be added on the same working day.



# DISTRIBUTION OF QUICKLIME

Be sure to  
add enough  
water to  
hydrate/slake  
pebbles before  
mixing



# SPREADING LIME SLURRY

BMP's for  
controlling  
stabilization  
limits





# LIME SLURRY BEING APPLIED



# INITIAL MIXING



# PULVERIZATION AND MIXING

- Two-stage
  - preliminary - distribute and wet
    - mellowing period – Refer to TxDOT spec for Item 260
  - final gradation:
    - 100% passing 1.75-in.
    - 85% passing 3/4-in.
    - 60% passing No. 4
- Mix 3% to 5% above optimum moisture content (BMP or TxDOT Spec)



# ADJUSTMENT MOISTURE CONTENT

Mix in lime using mixer



# MOISTURE CONTENT ADJUSTMENT



**Add more water if needed**



## “MELLOWING PERIOD” (1)

- Allows for cationic exchange and formation of initial pozzolanic reaction products
- “Breaks down” clay particles
- Optimized at 3-5 % over optimum moisture of treated soil. (see TxDOT spec for requirement)



## “MELLOWING PERIOD” (2)

- Minimum of 24 hours for Slurry, preferably overnight mellowing. (see TxDOT Spec for requirement).
- 48 Hours or more for dry placed quicklime or higher P.I. Clays with either slurry or dry.



## Importance of Mix Depth & Mellowing

- Depth Of Mixing
- Mellowing – Need to have the time for lime to have thorough contact with clay layers (Remember Huge Surface Area)
- Moisture Control – Water is needed for chemical reaction between lime and soil. Use more water if using quicklime.





# UNIFORMITY OF MIXING & PARTICLE SIZING

## After Mellowing

- Additional water
- Additional mixing
- check mixing depth, homogeneity (using phenolphthalein in field or pH test in lab)
- check particle sizing



## INSPECTION OF STABILIZATION



- Dig small hole and test with phenolphthalein pH solution in loose or compacted material
- Spray solution along face of hole to determine depth of treatment (soil will turn pink color at a pH 9 or higher)
- Measure compacted depth using Tex-140-E

# REMIXING



After  
mellowing

Reworking  
treated soil

- Remix soil-lime mixture after mellowing period to achieve gradation
- Moisture condition to a minimum +2 above OMC (or TxDOT spec) prior to compaction

## COMPACTION (1)

- Prior to mellowing intermediate light compaction - seal to reduce carbonation
- 95% of Maximum Dry Density in accordance with Tex-121-E or as specified
- Compact as soon as possible after mixing
- Most common to compact in one lift with a padfoot followed by a pneumatic roller

# COMPACTION (2)



## COMPACTION (3)



Remember:

Density (dry unit weight) of treated soil will be lower than native soil

Optimum moisture content of treated soil will be higher than native soil

## CURING (1)

- After mellowing and compaction
- Additional water
- Prevents stabilized section from drying out
  - prevents cosmetic shrinkage cracking
  - allows progress of pozzolanic reaction



Moisture Cured Subgrade

## CURING (2)

- During curing period
  - emulsion curing seal
  - aggregate Base Course section





## BENEFITS OF SOIL STABILIZATION

- ▶ Eliminates excavation & disposal costs
- ▶ Provides a stable & uniform layer or paving structure
- ▶ Decreases expansion/swelling
- ▶ Improves foundation support
- ▶ Achieves long term strength & performance

# QUESTIONS?

